Effect of Carbonation on Mechanical Properties & Durability of Concrete with Replacement of Sand by Pond Ash

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Abstract - Effect of carbonation on mechanical properties and durability of concrete has been studied in this work. Concrete has been prepared using ordinary Portland cement as binder with water/binder ratio 0.45, 0.50 and 0.55. Compressive strength & electrical resistivity test have been preformed for noncarbonated & carbonated samples. Natural sand has been replaced with industrial by product (fly ash) for sustainable construction. All results have been compared with standard results available. Carbonation test indicates on improvement of strength and electrical resistivity.

Keywords: Compressive strength, electrical resistivity, accelerated carbonation testing method, lastingness

I. INTRODUCTION

Ponded ash (PA) is waste product of thermal power plant, obtained as wet disposal of fly ash. Its major constituents are CaO, MgO and SO$_3$. Large land areas, near thermal power plants degraded due to its dumping. Ponded ash proved as sand replacement for sustainable and durable construction up to 50% replacement level [1, 2, 3, and 4]. It is also observed that use of PA as fine aggregate replacement improves shrinkage properties in fresh and hardened state [5]. Ponded ash can be treated as low reactive material which loses its pozzolanicity and hence suitable as fine aggregate replacement [6]. Carbonation improves the mechanical properties of concrete and reduces the chloride ion permeability but increases the corrosion rate of rebar in reinforced concrete structures (RCC) [7]. Corrosion rate of rebar’s encased under concrete can be measured using electrical impedance measurement [8].

Carbonation of concrete can be explained as the process by which atmospheric carbon di oxide (CO$_2$) in presence of moisture changes into dilute carbonic acid (HCO$_3$). It attacks on normal system of concrete and reduces alkalinity which provides protection to the reinforcement in RCC. This process is responsible for corrosion of reinforcement.

This work may justify the use of ponded ash in concrete suitable for the areas around thermal power plants. There are no any literatures available on carbonation exposuer by replacement of fine aggregate with pond ash as per knowledge of authors. Hence, this work gives courage to work on sustainable construction.

II. METHODOLOGY

A. Material characterization

OPC confirming to ASTM C 150-02 (ASTM 202e) [9] was used as cement. The fine aggregates were natural river sand with specific gravity 2.55, water absorption 1.01%, fineness modulus 2.84 and PA with specific gravity 2.02, water absorption 2.29% and fineness modulus 2.7 confirming to Zone 2 of BS 882 [10]. The coarse aggregates were nominal maximum size of 20 mm with specific gravity 2.73, water absorption 0.56% and fineness modulus 7.17 used in this work confirming to BIS 2386 (III)-1963 [11].

B. Experimental programme

All the mixes were designed as per BIS 10262-2009[12] and designated as shown in table 1.

<table>
<thead>
<tr>
<th>Mix No</th>
<th>Mix Type</th>
<th>w/c</th>
<th>Cement (kg/m$^3$)</th>
<th>Fine Aggregate(kg/m$^3$)</th>
<th>Coarse Aggregate (kg/m$^3$)</th>
</tr>
</thead>
<tbody>
<tr>
<td>OPC</td>
<td>OPC</td>
<td>0</td>
<td>350</td>
<td>714</td>
<td>0</td>
</tr>
<tr>
<td>55</td>
<td>SA</td>
<td>.55</td>
<td></td>
<td></td>
<td>1142</td>
</tr>
<tr>
<td>OPCP</td>
<td>OPC</td>
<td>0</td>
<td>350</td>
<td>316</td>
<td>31</td>
</tr>
<tr>
<td>55</td>
<td>(PA+SA)</td>
<td>.55</td>
<td></td>
<td></td>
<td>6</td>
</tr>
<tr>
<td>OPCP</td>
<td>OPC</td>
<td>0</td>
<td>384</td>
<td>311</td>
<td>31</td>
</tr>
<tr>
<td>50</td>
<td>(PA+SA)</td>
<td>.50</td>
<td></td>
<td></td>
<td>1</td>
</tr>
<tr>
<td>OPCP</td>
<td>OPC</td>
<td>0</td>
<td>427</td>
<td>305</td>
<td>30</td>
</tr>
<tr>
<td>45</td>
<td>(PA+SA)</td>
<td>.45</td>
<td></td>
<td></td>
<td>1100</td>
</tr>
</tbody>
</table>

All the mixes were prepared using lab purpose concrete mixers in standard manner, filled in 100x100x100 mm moulds in three layers and vibrated on vibrating table maintaining similar atmospheric condition. After 24 hrs of casting samples were de-moulded and kept for 28 days submerged curing in tank. Cubes were stored in controlled laboratory room, maintaining...
temperature range 27± 2º C and 65± 5% relative humidity for further testing after removal from curing tank. After 15 days of stabilization [13] three samples of each mix were kept in accelerated carbonation chamber for four month exposer in controlled environment of 10% CO₂, 27± 2º C temperature and 65± 5% relative humidity. The results of carbonated concrete is compared with non carbonated concrete of same duration (i.e. 0.25, 1, 2, 3 and 4 months) from the date of CO₂ exposer during testing of compressive strength and electrical resistivity. Carbonation depths were determined by spraying phenolphene indicator as per RILEM CPC 18 [14] on freshly broken cubes through compression testing machine. Electrical resistivity measurements were taken before strength test on saturated surface dry samples as per guideline of Giatec RCON2 [15].

III. RESULTS AND DISCUSSIONS

A. Compressive strength

Compressive strength test of carbonated concrete after 1 to 4 month were tested along with non carbonated concrete sample of similar duration. Figure 1 shows relation between average compressive strength of non carbonated and carbonated concrete. The variation in compressive strength compared with non carbonated control concrete is shown in figure 2.

![Fig. 1 Relation between average compressive strength of non carbonated and carbonated concrete.](image1)

![Fig. 2 The variation in compressive strength compared with non carbonated control concrete.](image2)

It is observed that carbonated sample shows higher compressive strength than non carbonated sample concrete. The variation in strength is ranges between 7% in higher water/cement ratio to 35% in lower w/c ratio, in the samples with pond ash as fine aggregate replacement. The R² value in the range 0.88% to 0.94% indicates strong correlation with the obtained results.

The increase in strength is due to the formation of solid calcium carbonate in the process of carbonation on replacement of calcium hydroxide.

This is similar to the results obtained by many researchers [5, 7, 13 and 16].

B. Electrical resistivity

Average electrical resistivity of concrete in non carbonated and carbonated stage is shown in figure 3. It is noted that carbonated electrical resistivity is very high on comparison with non carbonated control mix shows (in figure 4) 2.2 times to 3.4 times in control mix in carbonated concrete upto 4th month in very fast manner with R² value of 0.88. Concrete with low water cement ratio shows increment in the range 3.5 to 3.9 times giving R² value of 0.87. However, other two mixes gives non relevant statistical value hence discarded.

![Fig. 3 Relation between carbonated and non carbonated concrete](image3)

![Fig. 4 Variation of electrical resistivity of carbonated concrete on comparison with non carbonated control mix.](image4)

Looking to the obtained results of bulk electrical resistivity of concrete and relation with chloride permeability as in Table 2 we find that even the results of non carbonated concrete are showing moderate to less permeability for chloride penetration [17].

Pond ashes in concrete filled the pores and hence make the concrete more durable in lower w/c ratio sample.
TABLE II
RAPID CHLORIDE PERMEABILITY AND BULK ELECTRICAL RESISTIVITY

<table>
<thead>
<tr>
<th>Chloride penetration</th>
<th>56 Days rapid chloride permeability Charge passed per ASTM C 1202 (Coulombs)</th>
<th>28 Days Bulk electrical resistivity of standard concrete (kΩ.cm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>High</td>
<td>&gt;4000</td>
<td>&lt;5</td>
</tr>
<tr>
<td>Moderate</td>
<td>2000-4000</td>
<td>5-10</td>
</tr>
<tr>
<td>Low</td>
<td>1000-2000</td>
<td>10-20</td>
</tr>
<tr>
<td>Very Low</td>
<td>100-1000</td>
<td>20-200</td>
</tr>
<tr>
<td>Negligible</td>
<td>&lt;100</td>
<td>&gt;200</td>
</tr>
</tbody>
</table>

C. Carbonation depth
The figure 5 shows carbonation depth at various ages for the given concrete. On observation of the results of carbonation depth it is clear that carbonation depth increases in concrete with pond ash which is matching with the results as in figure 1 and 3 i.e., compressive strength and bulk electrical resistivity is highest in carbonated concrete.

![Fig. 5 Carbonation depths at various ages for the given concrete.](image)

D. Morphological detailing
Field Emission Gun Scanning Electron Microscopy (FEG SEM) results of the concrete with PA in carbonated and noncarbonated conditions are as under in figure 6 and 7;

![Fig. 6 OPC PA carbonated concrete in four months](image)

![Fig. 7 OPC SA carbonated concrete in four months](image)

IV. CONCLUSIONS
Utilization of Pond Ash as fine aggregate replacement can be a better option for sustainable construction with following benefits;
- Higher compressive strength
- Higher bulk electrical resistivity
- Less permeable concrete
- Denser structure is the result of PA inclusion Fig 6, and 7.
- Economy in construction as pond ash is available as waste material needs disposal to save the environment

ABBREVIATIONS
NOPC and COPC : Non Carbonated and carbonated concrete
NOPCP 55 and COPCP 55 : Non Carbonated and carbonated concrete with w/c ratio 0.55 with PA
NOPCP 50 and COPCP 50 : Non Carbonated and carbonated concrete with w/c ratio 0.50 with PA
NOPCP 45 and COPCP 45 : Non Carbonated and carbonated concrete with w/c ratio 0.45 with PA

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REFERENCES


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